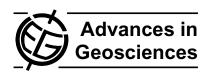
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Regional analysis of runoff thresholds behaviour in Southern Italy based on theoretically derived distributions

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Abstract. The analysis of runoff thresholds and, more in general, the identification of main mechanisms of runoff generation controlling the flood frequency distribution is investigated, by means of theoretically derived flood frequency distributions, in the framework of regional analysis. Two nested theoretically-derived distributions are fitted to annual maximum flood series recorded in several basins of Southern Italy. Results are exploited in order to investigate heterogeneities and homogeneities and to obtain useful information for improving the available methods for regional analysis of flood frequency.

1 Introduction

Inferring the flood frequency distribution in ungauged basins is one of the most important goals in the hydrological applied research. In many areas of the world, as well as in some parts of Southern Italy, the absence of reliable discharge measurements or the limited length of observed flood time series still imposes to determine the design floods through the statistical analysis of rainfall distribution coupled with deterministic rainfall-runoff models. This choice is often facilitated by the availability of more dense networks and long lasting records of rainfall measurements. Besides this, since the years '80s and '90s in Southern Italy regional analysis for flood prediction and flood frequency analysis has been applied, using methods based on the index flood approach and regional growth curves on homogeneous areas (e.g. Rossi and Villani, 1992). Today, this kind of approach still represents the most robust methodology for flood prediction in ungauged basins but it shows two main strong limitations. The first one, which heavily affects the accuracy of prediction, is related to the in-



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trinsic difficulty to find reliable and robust relationships for the index flood estimation based on available physical measures of basin features. The second one is related to the use of homogeneous areas whereas the physical spatial variability affecting the flood frequency distribution is neglected within homogeneous areas. The physical spatial variability of parameters, usually, is hard to detect by means of the available datasets of annual maximum series of flood peaks (AMFS), being masked by high sample variability. In this research we try to tackle this problem by analyzing, at the regional scale, the behavior of some key parameters of theoretically derived distributions. We use two different theoretically derived distributions, the IF (Iacobellis and Fiorentino, 2000) distribution and the TCIF (Two Component IF) distribution (Gioia et al., 2008). The two distributions are "nested" because the IF is a particular case of the TCIF distribution. In particular the TCIF is able to represent the occurrence of ordinary and extraordinary floods: when the second of these components is not present the IF is applied. The IF distribution has been already applied to basins of Puglia and Basilicata (Fiorentino and Iacobellis, 2001; Fiorentino et al., 2007), respectively South-Eastern and South-Central regions of Italy, while the TCIF distribution has been tested (so far) only in ten highly-skewed basins of Puglia, Basilicata and Calabria (South-Western region). In this paper we report results of the application of these distributions to thirty-three basins of the three regions and provide useful insights for the regional estimation of their parameters. The used AMFS cover a wide range of natural basins, different for climate, geomorphology, vegetation coverage, soil type and permeability.

The overall procedure is composed of three steps. First, we report values of all those parameters of the IF and TCIF distributions which are estimated without using AMFS. Among these are all (local and regional) parameters related to precipitation. These parameters are obtained by regional studies performed on annual maxima rainfall series. In the second step, the remaining unknown parameters are calibrated using

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AMFS, and the best distribution is selected between IF and TCIF. Being both distributions physically based, this procedure allows one to identify how many and which are the main components of the rainfall-runoff process playing as sub-models of the theoretical distribution. Finally, heterogeneities and homogeneities are analyzed in terms of spatial variability of the calibrated parameters.

2 Case studies and application

We applied the IF and TCIF distributions to thirty-three gauged basins in Southern Italy shown in Fig. 1 where the regions of Puglia, Basilicata and Calabria are highlighted. This sample of basins is extremely interesting for the variety of attributes that it displays (see Table 1). Basins have area ranging from 15 to $1657 \, \mathrm{km^2}$ and the series of annual maximum floods are characterized by highly variable skewness coefficient (0.08 < Cs < 3.18). The mean annual rainfall ranges from 600 mm in Puglia to more than 1800 mm in some parts of Basilicata and Calabria. The basin average Thornthwaite climatic index *I* (e.g. Thornthwaite, 1948), varies from -0.28 in Puglia, corresponding to *semi-arid* climate, to 1.66, corresponding to *hyper-humid* climate, in Calabria.

3 Model description and parameter estimation

3.1 The IF and TCIF distributions

The IF distribution (Iacobellis and Fiorentino, 2000) assumes that the exceedance probability function of the peak direct streamflow Q, $G_Q'(q)$, is derived integrating, over the appropriate domain, the joint probability density function (pdf) of two stochastic variables (e.g. Eagleson, 1972): the source area contributing to runoff peak a (whose probability density function is assumed Gamma distributed with position parameter depending on the ratio, r, of the expected contributing area to the basin area) and the runoff peak per unit of contributing area, u_a . Both random variables are controlled by rainfall intensity, duration and areal extension, runoff concentration and hydrological losses.

The runoff peak per unit area, u_a , is linearly dependent on the areal rainfall intensity (which is assumed Weibull distributed with shape parameter k) in a time interval equal to lag-time (τ_A) with a constant routing factor equal to 0.7. The IF distribution assumes that both average rainfall intensity ($E[i_{a,t}]$) and average hydrologic loss (f_a) scale with contributing area a according to the following power law relationships:

$$E[i_{\mathbf{a},\tau}] = E[i_{\mathbf{A},\tau}] (a/A)^{-\varepsilon} \quad f_{\mathbf{a}} = f_{\mathbf{A}} (a/A)^{-\varepsilon'}$$
 (1)

where $E[i_{A,\tau}]$ and f_A are the average rainfall intensity and the average hydrologic loss referred to the entire basin area A, while ε and ε' are scaling exponents. Thus, under

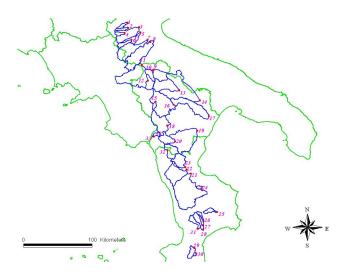


Fig. 1. Investigated basins in Southern Italy.

the hypothesis of Poissonian occurrence of independent annual maximum floods, the cumulative distribution function $F_{Q_p}(q_p)$ of the annual maximum flood peak $q_p = Q + q_o$, is derived, with q_o the base flow. Moreover the following relationship between the mean annual number of rainfall (Λ_p) and flood events (Λ_q) holds:

$$\Lambda_{\mathbf{q}} = \Lambda_{\mathbf{p}} \exp\left(-f_{\mathbf{A}}^{k} / E[i_{\mathbf{A},\tau}^{k}]\right). \tag{2}$$

Starting from the consideration that different mechanisms may arise, in any basin, with different frequency and magnitude (e.g., Sivapalan et al., 1990), Gioia et al. (2008) generalized the IF theoretical probability distribution introducing a two-component derived distribution called "Two Component IF" distribution (TCIF). In this new framework two different response types linked to different runoff thresholds-driven processes are identified:

- "L-type" (frequent) response, occurring when a lower threshold $f_{a,L} = f_{A,L} (a_L/A)^{-\varepsilon_L}$ is exceeded, and responsible of ordinary floods likely produced by a relatively small portion of the basin a_L .
- "H-type" (rare) response, occurring when a higher threshold $f_{a,H} = f_{A,H} (a_H/A)^{-\varepsilon_H}$ is exceeded, and providing extraordinary floods mostly characterized by larger contributing areas a_H .

The flood-peak contributing areas a_L and a_H are assumed, in analogy with the IF distribution, as Gamma distributed with expected contributing area respectively equal to $r_L A$ and $r_H A$.

In both mechanisms, the runoff threshold f_a is described by a simple power law with exponent ε' as in Eq. (1). Fiorentino and Iacobellis (2001) showed that such exponent provides an important signature of the expected behavior of

Table 1. Main features of the investigated basins.

	n.	Sample Size	$A (\text{km}^2)$	Cs	Ι	r	$r_{ m L}$	$r_{ m H}$	f _A (mm/h)	f _{AL} (mm/h)	f _{AH} (mm/h)	$\Lambda_{ m q}$	$\Lambda_{ m L}$	Λ_{H}	Best Fit
Santa Maria at Ponte															
Lucera Torremaggiore	1	15	58	0.99	-0.28	_	0.03	0.59	_	6.04	10.37	_	2.38	0.74	TCI
Triolo at Ponte Lucera															
Torremaggiore	2	16	56	0.45	-0.25	0.19	_	_	7.78	7.76	27.08	3.90	3.86	0.06	IF
Salsola at Ponte															
Foggia San Severo	3	40	455	0.28	-0.27	0.12	_	_	1.69	1.69	4.82	6.10	5.65	0.45	IF
Casanova at Ponte															
Lucera Motta	4	16	57	1.32	-0.14	_	0.1	0.72	_	5.31	11.23	_	3.61	0.61	TC
Celone at Ponte Foggia															
San Severo	5	39	233	2.43	-0.24	_	0.09	0.37	_	2.29	7.96	_	5.64	0.18	TC
Celone at San Vincenzo	6	15	92	1.27	-0.06	0.11	_	_	3.12	3.12	15.47	7.10	7.04	0.06	IF
Cervaro at Incoronata	7	53	539	0.64	-0.19	0.62	_	_	1.76	1.76	17.28	5.50	5.52	0.00	IF
Carapelle at Carapelle	8	36	715	1.34	-0.23	_	0.41	0.99	_	1.01	3.86	_	9.86	0.66	TC:
Venosa at Ponte Sant' Angelo	9	34	263	2.26	-0.17	_	0.06	0.99	_	2.93	5.93	_	2.93	0.49	TC
Arcidiaconata at Ponte															-
Rapolla Lavello	10	32	124	0.85	-0.04	0.2	_	_	3.33	3.33	11.55	5.00	4.88	0.12	IF
Ofanto at Rocchetta															
Sant' Antonio	11	52	1111	0.46	0.16	0.9	_	_	1.12	1.12	15.80	4.80	4.82	0.00	IF
Atella at Ponte															
sotto Atella	12	45	176	0.96	0.17	0.16	_	_	1.99	1.98	32.15	5.60	5.62	0.00	IF
Bradano at Ponte															
Colonna	13	32	462	1.21	-0.08	_	0.15	0.99	_	2.01	5.08	_	3.98	1.04	TC
Bradano at San		5 2	.02	1.21	0.00		0.10	0.,,,		2.01	2.00		2.70	1.0.	
Giuliano	14	17	1657	1.03	-0.17	0.38	_	_	2.03	2.02	8.44	3.40	3.35	0.07	IF
Basento at Pignola	15	28	42	1.13	0.7	0.23	_	_	0.00	0.00	44.50	21.00	21.00	0.00	IF
Basento at Gallipoli	16	38	853	2.25	0.28	-	0.25	0.86	-	0.99	8.00	_	8.35	0.17	TC
Basento at Menzena	17	24	1382	1.57	0.08	0.2	-	-	1.32	1.32	9.04	6.20	6.13	0.07	IF
Agri at Tarangelo	18	32	511	1.19	0.47	0.19	_	_	0	0.00	8.16	21.00	20.93	0.07	IF
Sinni at Valsinni	19	22	1140	2.42	0.57	-	0.16	0.85	_	0.00	8.75		20.58	0.24	TC
Sinni at Pizzutello	20	17	232	0.82	1.26	0.3	-	-	0	0.00	4.04	32.00	27.29	4.71	IF
Crati at conca	21	31	1339	1.44	0.61	0.12	_	_	0.00	0.00	65.02	15.00	20.00	0.00	IF
Esaro at La musica	22	19	520	1.64	0.77	0.12	0.2	0.6	- 0.00	0.00	9.67	13.00	17.32	0.30	TC
Coscile at Camerata	23	29	285	1.65	0.65	0.08	-	-	1.18	1.18	96.12	6.10	6.12	0.00	IF
Trionto at Difesa	24	16	32	3.18	0.03	0.00	0.03	0.14	1.10	1.65	40.49	0.10	6.84	0.06	TC
Tacina at Rivioto	25	25	79	2.79	1.43	_	0.03	0.14	_	1.41	21.78	_	4.25	0.37	TC
Alli at Orso	26	47	46	2.79	1.43	0.04	0.08	0.43	0.56	0.56	171.99	11.10	11.10	0.00	IF
Melito at Olivella	27	16	41	1.42	0.72	0.04	_	_	0.00	0.00	163.47	20.00	20.00	0.00	IF
Corace at Grascio	28	38	182	1.42	0.72	0.03	_	_	0.00	0.32	116.14	11.70	11.70	0.00	IF
Ancinale at Razzona	28 29	50	116	1.34	1.34	0.23	_	_	0.32	0.32	220.86	7.60	7.62	0.00	IF IF
Ancinale at Kazzona Alaco at Mammone	30	50 19	116	1.34	1.54	0.09	_	_	0.19	0.19	417.53	10.00	10.00	0.00	IF
Alaco at Mammone Amato at Marino	31	19 26	113	2.43	0.86	0.05	0.12	0.79		1.55	417.53 11.67	10.00	4.90	0.00	TC
							0.12	0.79	0.10			21.00			
Lao at piè di Borgo	32	24	280	0.98	1.16	0.21	_	_	0.19	0.19	92.58	21.00	21.02	0.00	IF
Noce at la Calda	33	30	43	0.08	1.58	0.08	_	_	0.00	0.00	174.45	34.00	33.99	0.00	IF

soil infiltration. In humid climates typically one finds $\varepsilon'=0$ that implies a constant infiltration rate, while $\varepsilon'=0.5$ is expected in dry climate where the threshold f_a shows a storage behavior. These two different mechanisms were used by Gioia et al. (2008), working on a dataset that included two dry cases out of ten basins. They found $\varepsilon_L=0$ and $\varepsilon_H=0.5$ thus associating a constant infiltration behavior for the L-type response which develops in areas closer to the river network, and a storage behavior for the H-type response arising in areas far from the river network and, then, characterized by lower soil moisture in conditions antecedent the rainfall events.

Assuming that L-type and H-type events are independent and that both rates of occurrence are Poisson distributed, the overall process of exceedances is also a Poisson process and the cdf of the annual maximum floods is

$$F_{Q_p}(q_p) = \exp\{-\Lambda_L[G_{Q,L}'(q_p)] - \Lambda_H[G_{Q,H}'(q_p)]\},$$
 (3)

where $G_{Q,L}$ and $G_{Q,H}$ are the peak flow distributions corresponding to components of L-type and H-type events; Λ_L and Λ_H are respectively the mean annual number of independent flood events for L-type and H-type processes and are related to the runoff thresholds by means of the following relationships:

$$\Lambda_{\rm q} = \Lambda_{\rm L} + \Lambda_{\rm H} = \Lambda_{\rm p} \exp\left(-\frac{f_{\rm A,L^k}}{E[i_{\rm A,\tau^k}]}\right) \tag{4a}$$

$$\Lambda_{\rm H} = \Lambda_{\rm p} \exp\left(-\frac{f_{\rm A,H}^k}{E[i_{\rm A,\tau^k}]}\right),\tag{4b}$$

The IF and the TCIF distributions are characterized respectively by twelve and fifteen parameters (see Gioia et al., 2008); nevertheless, for their estimation, based on current knowledge, only four parameters of the TCIF, $r_{\rm L}$, $r_{\rm H}$, $\Lambda_{\rm H}$, $\Lambda_{\rm L}$, and two parameters of the IF, $r_{\rm L}$ and $\Lambda_{\rm q}$, require to be calibrated by using the recorded AMFS. In facts, all the remaining parameters can be retrieved from information other than AMFS and principally from rainfall regional datasets.

3.2 Parameter estimation and results

The parameters estimated without using AMFS are common to the IF and TCIF distributions and were all available from previous studies (see Gioia et al., 2008, Fiorentino and Iacobellis, 2001; Iacobellis and Fiorentino, 2000; Claps et al., 2000).

The loss threshold scaling factor of the IF distribution ε was assumed, following Fiorentino and Iacobellis (2001), $\varepsilon' = 0$ for humid basins (I > 0) and $\varepsilon' = 0.5$ for basins characterized by dry climate (I < 0). The two loss threshold scaling factors ε_L and ε_H of the TCIF were estimated following the regional procedure performed by Gioia et al. (2008). This procedure is based on the structural similarity of the TCIF and TCEV distribution (Rossi et al., 1984). Both models, in fact, arise as distributions of annual maxima of a Poisson compound process characterized by two independent base processes. Then, the TCEV includes two parameters Λ_1 and Λ_2 which represent the mean annual number of events of the ordinary and extraordinary components, respectively. Assuming, for one moment, that the L-type and H-type events correspond to the TCEV ordinary and extraordinary components, Λ_1 and Λ_2 can replace Λ_L and Λ_H in Eq. (4). Thus, from the available AMFS, we evaluated the at-site TCEV-ML (Two Component Extreme Value- Maximum Likelihood) estimates of Λ_1 and Λ_2 and assuming $\Lambda_L = \Lambda_1$ and $\Lambda_H = \Lambda_2$ we obtained some approximated estimates of $f_{A,L}$ and $f_{A,H}$, using Eq. (4) re-written as:

$$f_{A,L} = \left[E[i_{A,\tau}^k] \log \left(\frac{\Lambda_p}{\Lambda_L + \Lambda_H} \right) \right]^{1/k}$$
 (5a)

$$f_{A,H} = \left[E[i_{A,\tau}^k] \log \left(\frac{\Lambda_p}{\Lambda_H} \right) \right]^{1/k}.$$
 (5b)

where k, Λ_p , $E[i_{A,\tau}]$ were estimated from the annual maximum rainfall series.

Observing the regional behavior of such estimates we found that for all basins, independently from climate, the higher threshold $f_{\rm A,H}$ scales with exponent $\varepsilon_{\rm H}=0.5$. The lower threshold, on the other hand, showed a strong dependence on climate being $\varepsilon_{\rm L}=0$ in humid basins (I>0) and $\varepsilon_{\rm L}=0.5$ in dry basins (I<0).

Then we used the AMFS for the estimation of parameters Λ_q and r of the IF distribution and Λ_L , Λ_H , r_L , r_H of the TCIF distribution adopting the at-site evaluation procedure which is described in Iacobellis et al. (2010), where for each river basin, the best parameter-dataset is chosen as the one maximizing the log-likelihood function of the observed sample of annual maximum floods. The selected parameterdatasets are reported in Table 1 for all basins while in Fig. 2 we display the TCIF-cdf, the IF-cdf and the Weibull plotting positions of the AMFS of the thirty-three basins in a Gumbel probability plot. It is worth noting that for twentyone out of thirty-three basins the IF distribution was selected as the best-fitting while the others are TCIF distributed. Iacobellis et al. (2010) tested and discussed the suitability of different selection criteria based on the maximization of the log-likehood function and accounting for model parsimony, by using a cross-validation technique for benchmark. They concluded, accordingly with Busemeyer and Wang (2000), that the use of a log-likelihood criterion, without any penalty factor accounting for model parsimony, is suggested when dealing with the IF and TCIF nested distributions and small sample size. It is also important to mention that, being the IF and TCIF nested distributions, the IF is selected whenever the parameter estimation procedure of the TCIF provides values of Λ_H which tend to 0, meaning that the TCIF curve collapses into the IF curve. From the analysis of data in Table 1 it is possible to observe that many of the TCIF-distributed series are characterized by a high skewness coefficient, but this is not a general rule. These results confirm that the presence of two different mechanisms of runoff generation may lead to highly-skewed distribution, nevertheless it is also possible to observe basins with two components and not particularly high skewness coefficient and it is also possible to have a high skewness coefficient in basins characterized by a single runoff component.

In Fig. 3 we show the scaling relationship of the runoff thresholds for all river basins: the red markers represent the values of $f_{A,H}$ for dry basins (stars) and humid basins (squares); in blue we report the $f_{A,L}$ values (stars) and the f_A values (circles) observed in dry basins; in green are shown the $f_{A,L}$ values (squares) and the f_A values (circles) observed in humid basins. Four different patterns can be distinguished related to the considered components and the climatic conditions. These patterns are highlighted by the continuous lines representing power laws with exponent 0.5 (blue, red and magenta) and 0 (green). These scaling relationships are consistent with the behaviors above discussed for the scaling factors ε , ε _L and ε _H. More important, they demonstrate that when comparing dry and humid basins the behavior of

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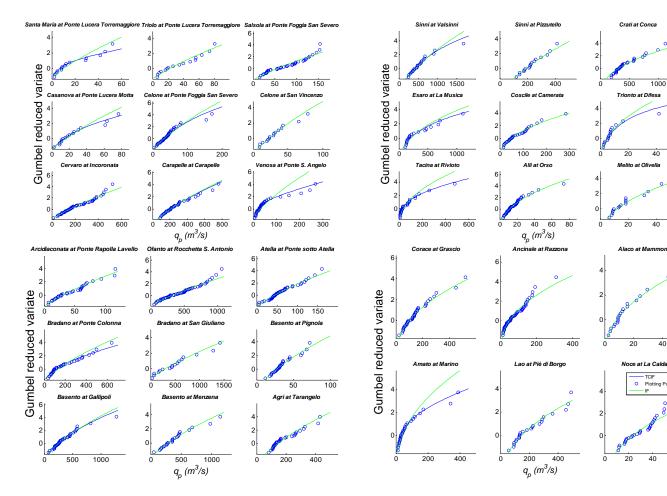


Fig. 2a. Probability plots of IF and TCIF distributions vs. AMFS – plotting positions.

Fig. 2b. Probability plots of IF and TCIF distributions vs. AMFS – plotting positions.

the lower threshold is still different, although in both cases it is quite homogeneous and independent on the presence of a second higher threshold. The scaling behavior of the second higher threshold, with exponent 0.5 denotes a capacitive behavior related to water storage in the soil-vegetation package in both humid and dry basins. Nevertheless the soil storage capacity is higher for humid basins than for the dry ones. Also, for humid basins the first runoff threshold is identified as a constant infiltration rate (which may be related to the shallow groundwater drainage) while in dry basins it corresponds to a water storage capacity whose nature will be discussed in conclusions.

Conclusions

The performances of two nested theoretically derived distributions, IF and TCIF, were tested on several heterogeneous river basins of Southern Italy. Results show that, despite the statistical homogeneity which was claimed over the entire region using the flood index method (e.g. Fiorentino et

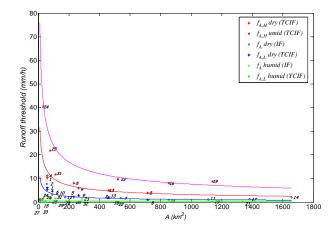


Fig. 3. Regional scaling of different runoff thresholds.

al., 1987), a significant spatial variability can be detected by means of theoretically derived distributions. Almost half of the basins, and precisely twelve out of thirty-three, show evidence of two different mechanisms of runoff generation. Results show that there is not a direct relationships between the presence of two components and a high skewness coefficients. Dynamics of different processes are summarized in Fig. 3 where the average hydrological losses during a flood event are plotted as a function of the basin area. This graph depicts the characteristics of different mechanisms of runoff generation. The behavior of humid basins was already assessed in other papers and in particular Gioia et al. (2008) recognized a saturation excess mechanism as responsible of the lower threshold and an infiltration excess mechanism for the higher threshold. On the other hand different interpretations and different real situations are feasible in dry basins. Results indicate that two different capacitive thresholds are responsible, in dry basins, for the lower and higher thresholds. These may be due to different patterns of vegetation, soil, geology and geomorphological factors. Also the presence of a saturation excess mechanism can be included among possible causes, although this one should be distinguished from the one occurring in humid climate. In humid climates the saturation excess mechanism occurs as a consequence of the watertable rise which is seasonally recharged. In dry basins this kind of mechanism may still arise when a shallow groundwater, originated by an impermeable bedrock layer underlying the river network, is filled during rain events. This conceptual interpretation is also consistent with the conceptual model developed by McGrath et al. (2007) which use a threshold storage for the saturation excess process. Further light about the physical interpretation of the observed spatial variability of flood frequency distribution may arise from the analysis of the estimates of the average ratios of contributing areas r_L and r_H and of the residual, local variability of $f_{A,L}$ and $f_{A,H}$ which is still object of re-

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