



Accuracy of sand beach topography surveying by drones and photogrammetry

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Abstract

Beaches are characterized by high morphodynamic activity, and high-frequency measurements are needed to understand their states and rates of change. Ideally, beach survey methods should be at once accurate, rapid and low-cost. Recently, unmanned aerial systems (drones) have been increasingly utilized to measure beach topography. In this paper, we present a review of the state of art in drones and photogrammetry for beach surveys and the respective achieved measurement quality (where reported). We then show how drones with a minimal configuration and a low-cost setup can meet the high accuracy and rapidity required for beach surveys. To test a minimal drone and ground control point configuration, we used consumer-grade equipment to perform the same flight path with different cameras and at different altitudes. We then used photogrammetry to produce digital elevation models of the beach. Using a GNSS-RTK system, we collected 2950 independent control points to evaluate the accuracy of the digital elevation models. Results show that, once a few potential sources of uncertainties in the final digital elevation model are taken into account, the average RMSE(z) of the digital elevation models was ~5 cm, with a survey efficiency of ca. 3 m² min⁻¹. Digital elevation models taken at different times were used to calculate the before–after sediment budget following a storm that hit a sandy coast in Sylt Island at the German North Sea coast.

Introduction

Coastal environments and, in particular, beaches are rapidly evolving systems, driven by the continuous interaction of the topography and transport processes with wind, wave and tidal forcing. In order to understand and quantify coastal morphodynamics, it is necessary to acquire high-resolution data on beach topographic changes. The common in situ monitoring method for the measurement of beach topography (especially at small scales, up to few hundred meters of coastline) are cross-

shore GNSS-RTK (Global Navigation Satellite System–Real-time Kinematic) surveys, which can result in highly accurate information along the measured transects (Harley et al. 2010).

In the last decade, UAS (Unmanned Aerial System)-based photogrammetry has become a common tool for topographic surveying (Eisenbeiss 2009). This method has been employed in many environments, from polar to tropical regions (e.g. Dąbski et al. 2017; Ryan et al. 2015; Casella et al. 2017; Chirayath and Earle 2016). Recent studies have shown that the cost of acquiring topographic data with traditional in situ techniques is comparable with the cost of UAS-photogrammetry (Mancini et al. 2013). Thus, drones are rapidly becoming standard survey tools (Moloney et al. 2018): Once few simple steps are followed (see Figure 5 in Joyce et al. 2018), acquiring high-resolution data with drones is safe, straightforward and cost-effective.

Drones allow gathering multi-temporal data that can then be analyzed to assess the spatio-temporal evolution of erosional and depositional processes (e.g. Casella et al. 2016; see also Table 1). Beach environments, however, are particularly challenging for the photogrammetric methods as the low texture and contrast of the sand surface make it harder to match distinct features between different images (Eltner et al. 2015). As such, drone-based topographic reconstructions of beach

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Table 1 Review of papers where UASs have been used to study beach environments. The table includes the use made of drone data within the original paper (this field may contain simplifications of the original study), RMSE(z) computed with independent measurement (if available), number of independent measurement (if available) and the collection instrument(s) used to survey independent elevation data (if available). Legend: N.A. not available; *see Table 4 of this study for details. **Reported as standard deviation; ***Reported as median difference

Reference	Location	Beach type or description	Drone data usage	RMSE(z) [m]	Number of ICPs	ICP survey instrument
This study	Bunkerhill Beach, Sylt Island, Germany	Intermediate, medium to coarse sand beach with oblique sand bars	RMSE(z) assessment and Multi-temporal	0.052 ± 0.013* 0.051 ± 0.004*	2950 537	GNSS-RTK
Laporte-Fauret et al. 2019	Truc Vert, SW France	Beach-dune system (meso-macrotidal beach)	Multi-temporal	0.13 0.05	65	GNSS-RTK
Lowe et al. 2019	Sipadan Island, Sabah, Malaysia, and Sasahura Ite Island, Isabel Province, Solomon Islands.	Reef-island beach	Multi-temporal	0.06–0.43	From 1 to 11 per transects. Nb. of transects: 7 From 15 to 42 per transects.	Optical Level and GNSS (non-RTK)
Guisado-Pintado et al. 2019	Five Finger Strand, County Donegal, NW Ireland	Dissipative beach	Single survey	0.176**	Nb. of transects: 5	GNSS-RTK
Pitman et al. 2019	Amberley Beach, Pegasus Bay, New Zealand	Composite beaches	Single survey	0.02	N.A. 4 transects	GNSS-RTK
Taddia et al. 2019	Rosolina Mare, Rovigo, Italy	Embryo dunes	Multi-temporal	0.038**	~ 150	GNSS-RTK
Chen et al. 2018	Wujiao Bay, Dongshan Island, China	Sandy beach	Multi-temporal	0.11 0.11	73 110	GNSS-RTK
Conlin et al. 2018	South Beach State Park, Oregon, USA	Dissipative subaerial beach	Single survey	0.063 0.064	N.A. transects	GNSS-RTK
Duo et al. 2018	Ferrara, Emilia Romagna, Italy	Relict sandy beach ridges and artificial embankments	Storm assessment	0.12 0.14	~ 50 on Transect 1 ~ 50 on Transect 2	GNSS-RTK
Elsner et al. 2018	Pevensey Bay, East Sussex, UK	Composite mixed beach	Single survey	0.113	5613	wheel-based GNSS-RTK
Gonçalves et al. 2018	South of Figueira da Foz, Portugal	Sandy-shore exposed directly to highly energetic waves	Single survey	0.108	2417	ATV-based RTK-GNSS
Jeong et al. 2018	Gosapo beach, South Korea	Sandy beach face and low-relief tidal flat	Single survey	0.12	592,395	TLS
Ruessink et al. 2018	National Park Zuid-Kennemerland, the Netherlands	Frontal coastal dunes	Multi-temporal	0.026–0.033 0.029–0.039	N.A.	LIDAR
Talavera et al. 2018b	Camposoto beach, Cádiz, Spain	Intermediate-dissipative beach	Multi-temporal	0.084 0.107	147 262	GNSS-RTK
Talavera et al. 2018a	Sancti Petri spit, San Fernando, Cádiz, Spain	Intermediate-dissipative beach	Storm assessment	0.067	3191	GNSS-RTK
Seymour et al. 2017		Sandy beach	Single survey	0.05–0.08 0.055 0.069	~ 1500 ~ 1500	GNSS-RTK
				0.031–0.089	53	GNSS-RTK

Table 1 (continued)

Reference	Location	Beach type or description	Drone data usage	RMSE(z) [m]	Number of ICPs	ICP survey instrument
Matsuba et al. 2017	Bird Shoal and Bulkhead Shoal Beaufort, North Carolina	Sandy beach	Multi-temporal	0.097***	N.A.	GNSS-RTK
Pádua et al. 2017	Fukude Harbor, Shizuoka Prefecture, Japan Sandspit of Cabedelo, Douro estuary, Porto, Portugal	Sandspit	Multi-temporal	0.046 0.063	114 34	GNSS-RTK
King et al. 2017	Byron Bay, New South Wales, Australia	Sandy beach (urban area)	Single survey	0.068	73	GNSS-RTK
Sturdivant et al. 2017	Buzzards Bay, Massachusetts, USA	Shoreline, dune toe and dune crest	Single survey	0.087	253	GNSS-RTK
Turner et al. 2016	Narrabeen Beach, Sydney, Australia	Sandy open-coast embayment bound by rocky headlands (subaerial beach and dunes)	Storm assessment	0.068**	15,247	ATV-based RTK-GNSS
Casella et al. 2016	Borghetto Santo Spirito, Savona, Italy	Dissipative beach	Multi-temporal	0.15	254	GNSS-RTK
Long et al. 2016b	Bonne-Anse Lagoon-Inlet, La Palmyre, France	Estuary (sand bank part of the inlet flood delta)	Single survey	0.091	528	GNSS-RTK
Long et al. 2016a	Bonne-Anse Lagoon-Inlet, La Palmyre, France	Estuary (sand bank part of the inlet flood delta)	Multi-temporal	0.17 0.16	25	GNSS-RTK
Harwin et al. 2015	Pittwater Estuary, Tasmania, Australia	Eroding coastal scarp in a sheltered estuary	Single survey	0.005–0.02	34	Total station theodolite
Gonçalves and Henriques 2015	Cabedelo and Aguda beach, Porto, Portugal	Reflective (sand spit in the mouth of the Douro River)	Single survey	0.046	114	GNSS-RTK
Casella et al. 2014	Borghetto Santo Spirito, Savona, Italy	Dissipative beach	Storm assessment	0.12	23	GNSS-RTK
Mancini et al. 2013	Marina di Ravenna, Italy	Fore-dune (beach dunes system)	Single survey	0.19	126	TLS
Harwin and Lucieer 2012	Southeast Tasmania, Australia	Sheltered estuary	Single survey	0.11	~30	GNSS-RTK
Torres-Freyermuth et al. 2019	San Miguel, Teresiano, El Faro, (between Telchac and Progreso) Yucatán Peninsula, Mexico	Mild-slope continental shelf	Multi-temporal	0.04		GNSS-RTK
Sari et al. 2018	Wawaran Beach, Pacitan, East Java, Indonesia	N.A.	Single survey			
Addo et al. 2018	Volta river delta, Ghana	Delta region (sandy beach)	Storm assessment			
Guillot et al. 2018	Truc Vert Beach, SW France	Beach-dune system (meso-macrotidal beach)	Multi-temporal mix techniques (LIDAR-UAS)			
Moloney et al. 2018	St. Kilda, Dunedin, New Zealand	Foredune system (back beach, foredune stoss slope, lee slope, swale, bare sand, sparse and dense marram grass)	Multi-temporal			
Nikolakopoulos et al. 2017	Rio, Patraikos Gulf, Greece	Sandy beach	Single survey			

Table 1 (continued)

Reference	Location	Beach type or description	Drone data usage	RMSE(z) [m]	Number of ICPs	ICP survey instrument
Su and Gibeau 2017	Beach of South Padre Island, Texas Gulf Coast, Texas	Barrier island beach	Single survey			
Topouzelis et al. 2017	Lesvos, Greece	Swash zone, beach face, and big berm with sand dunes	Multi-temporal			
Scarelli et al. 2017	Ravenna, Emilia Romagna, Italy	Very dissipative beach	Multi-temporal			
Papakonstantinou et al. 2017	Lesvos, Greece	Swash zone, beach face, and big berm with sand dunes	Multi-temporal			
Scarelli et al. 2016	Capão Novo, Rio Grande Do Sul, Brasil	Dissipative and intermediate beach	Single survey			
Ierodiaconou et al. 2016	Adjacent to Warrambool Harbor, Victoria, Australia	Intermediate dissipative beach	Storm assessment			
Yoo and Oh 2016	Songjung beach, Korea	Sandy beach	Multi-temporal			
Papakonstantinou et al. 2016	Eressos and Neapoli beaches, Lesvos, Greece	Swash zone, beach face, and large berm with sand dunes (Eressos) and mixture of sand and gravel (Neapoli)	Single survey			
Lu 2016	Penghu islands, Taiwan	Beach embayment	Multi-temporal			
Dohner et al. 2016	Broadkill Beach, Delaware, USA	Sandy beach (tidal estuary)	Single survey			
Guillot and Pouget 2015	Oleron Island, France	Sandy beach (macrotidal environment)	Multi-temporal			
Chikhradze 2015	Aguçadoura and Ramalha beaches, Portugal	Sandy and shingle beaches with rocky outcrops, and dune systems	Single survey			
Harley et al. 2015	Narrabeen-Collaroy Beach, SE Australia	Embayed sandy beach	Storm assessment			

environments are often regarded as less accurate than those carried out in other geomorphic settings, such as geological outcrops or landslides.

In this study, first a short review on the quality of UAS beach topography measurements is presented. Second, for an exemplary beach domain, it is shown that, with minimal drone survey configuration, using low-cost equipment and small survey effort, it is possible to obtain highly accurate digital elevation models (DEMs) with an overall acquisition and elaboration efficiency of $3 \text{ m}^2 \text{ min}^{-1}$. This method is exemplified on datasets before and after a storm at Bunkerhill beach, Sylt Island, Northern Germany.

Literature review on drone-based beach topography

The use of UAS for the study of beach environments has increased exponentially in the last 5 years. In Table 1, we present a compilation of 47 papers where UAS were used to survey beaches. Overall, 52% of the works reviewed have been conducted in the European continent, the rest equally divided (15%) between the Australian, Asian and American continents. Only one study was conducted in Africa. The majority of the works focused on multi-temporal studies (about 60%,

Fig. 1 **a** Geographic location of the study area. **b** The beach area surveyed and the track of a typical flight of the drone. **c** Measurement of GCPs. **d** Measurement of ICPs, with RTK-GPS mounted on a sled



Table 2 Cameras used in this study

Camera	Field Of view	Megapixels	Resolution [pix]	Sensor size [mm]	Weight [g]
Canon PowerShot S100	71°	12	4000 × 3000	7.6 × 5.7	198
GoPro HERO4 Silver	171°	12	4000 × 3000	6.2 × 4.6	83
GoPro HERO4 Black	80°	12	4000 × 3000	6.2 × 4.6	88

including post-storm assessment studies) to monitor sediment displacement on the coast. 28 studies report the quality of the digital elevation models (DEM) derived from drone data and photogrammetry by comparison with independent measurements. In general, the instruments used to collect independent elevation measurements are mainly GNSS-RTK, LIDAR, TLS or total station. The highest accuracy (RMSE = 0.005 m) was achieved by using a total station to define the ground control points (GCP) for the calibration of the data and a limited set ($n = 34$) of independent control points (ICPs) for the quality assessment. Three studies used high-density point clouds (e.g. LIDAR and terrestrial laser scanning TLS) for comparisons. In those, RMSE ranged between 0.026 and 0.12 m. Almost 90% of the reviewed works used GNSS-RTK to measure ICPs. The number of ICPs varies among the different works, and only few of them (5 works) collected a high number of ICPs (more than 1000). Among these, the range of the accuracy of the DEMs varies between 0.05 and 0.11 m. The GNSS-RTK method to measure ICPs has an intrusive nature, which is discussed in “[Systematic errors on beach environments.](#)” The second part of Table 1 lists literature in which no assessment of accuracy of the DEM was carried out or not enough information was given if errors were based on independent measurements.

Study area and methods

The range of UAS topographic surveying quality was inter-compared with low-cost surveys carried out with different

settings and systems at the North Sea coast. The investigated area is a beach patch along the Bunkerhill beach-dune system, located in the southern part of the island of Sylt (Wadden Sea, North Germany; Fig. 1a). The surveyed beach area measures 120 m along-shore and 50 m cross-shore, extending from the upper part of the swash zone (2 m above mean sea level) to the foot of the dune (ca. 10 m above mean sea level) (Fig. 1b). To collect aerial data, we used a DJI Phantom 2 quadcopter. The approximate take-off weight of the UAS during the experiment was about 1300 g. We programmed a mowing-lawn flight plan into the native Phantom DJI app, “DJI Ground Station.”

The same flight path was performed (Fig. 1b) with different cameras and at different altitudes. A Canon S100 was used at altitudes of 30, 40 and 50 m above ground level. Two additional flights at 50 m above ground level were carried out with a GoPro Hero 4 Silver rolling shutter fisheye lens and a GoPro HERO4 Black modified to yield a similar field of view (FOV) as consumer-grade compact cameras (focal Length 5.4 mm, horizontal opening angle 60° and aperture range f/2.5). Specifications for the cameras used are given in Table 2. Flights were carried out at constant horizontal speed of 1.0 m/s, with track lines 12 m apart, resulting in effective overlaps indicated in Table 3 for each flight. Pictures were taken every 5 s for the Canon S100 and the GoPro Hero Silver and every 2.5 s for the GoPro Hero Black.

As none of the cameras was connected to the flight control, picture acquisition was triggered by time intervals. Therefore, the front and side overlap was planned before each survey on the basis of flight path (including flight altitude), geometry of camera sensor, focal length, photo frequency and speed of the aircraft. A minimum front overlap of 86% and side overlap of 67%

Table 3 Details on number of photos, effective overlap, tie points, DEM resolution, area covered and RMSE(z) resulting from each photogrammetric workflow. The RMSE(z) was calculated by Agisoft

Photoscan on the basis of GCP elevations. For detailed results for each flight, the reader is referred to the [Supplementary Materials](#)

Flight (camera and altitude)	Number of photos	Effective overlap	Tie points	DEM resolution [cm/pix]	Total area covered [m ²]	Photoscan RMSE(z) Z error [m]
Canon S100 (30 m altitude)	136	6.8	81,384	1.7	9290	0.011
Canon S100 (40 m altitude)	132	11.1	59,898	2.6	12,900	0.016
Canon S100 (50 m altitude)	137	11.5	45,305	3.0	14,600	0.016
GoPro Hero 4 Silver (50 m altitude)	104	11.6	29,667	4.0	49,200	0.006
GoPro Hero 4 Black (50 m altitude)	217	13.0	62,073	2.1	12,000	0.007

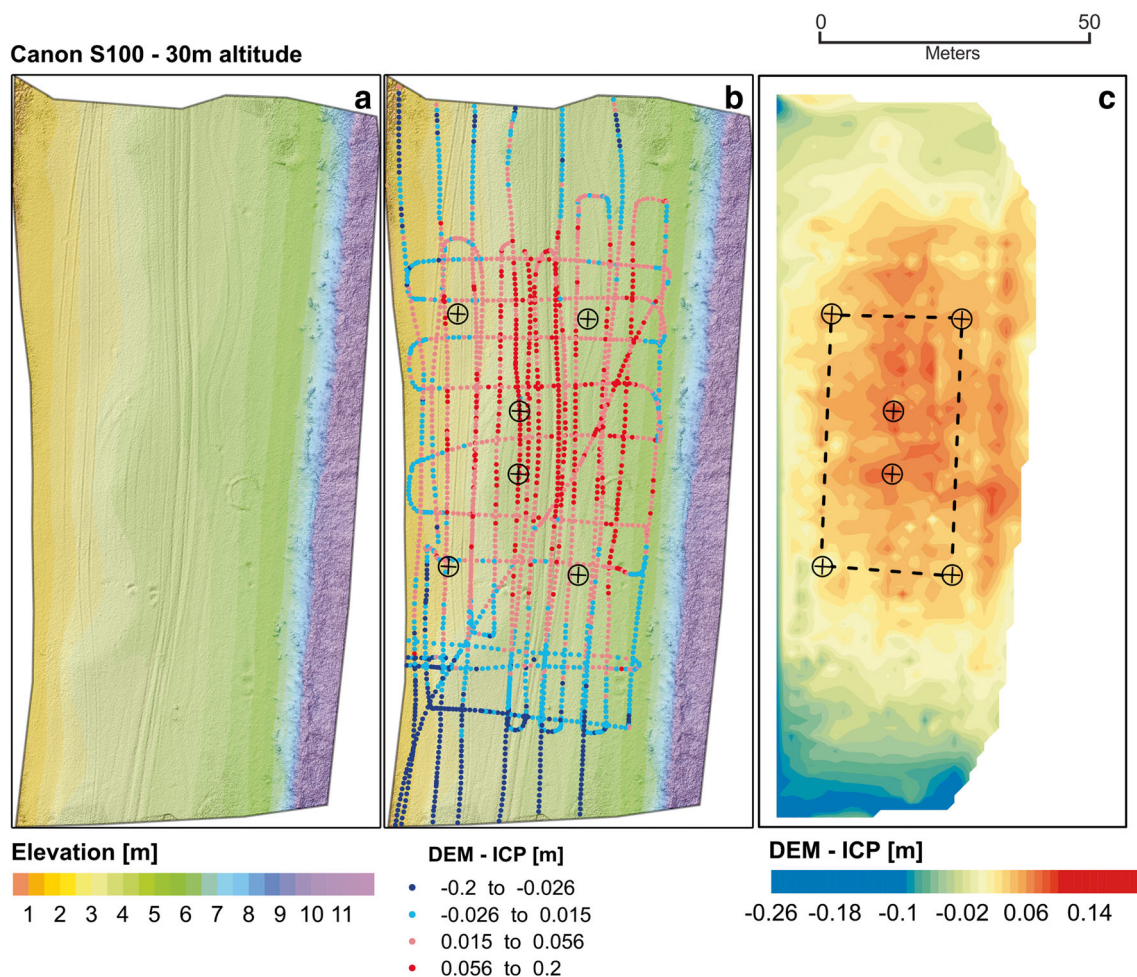


Fig. 2 **a** Digital elevation model (DEM) of the surveyed beach section. **b** Location of ground control points (GCPs, black crossed circles) and independent control points (ICPs, small dots). The color of the ICPs

relates to the difference between the elevation of the DEM and the elevation of the ICP. **c** Interpolated surface of the DEM-CP differences, created with natural neighbor algorithm

(worst case, Canon Powershot S100 flown at 30 m altitude) and a maximum front overlap of 90% and side overlap of 80% (best case, Canon Powershot S100 flown at 50 m altitude) was set.

Ground control points were measured using a JAVAD SIGMA/GrAnt G3T GPS utilizing real-time kinematic correction from an NTRIP server (HEPS service by SAPOS©). The final vertical accuracy of the GNSS points is in the range of 2–3 cm. Six ground control points (hereafter GCPs) were placed on a 25 by 40 m area and measured using RTK GPS mounted on a pole (Fig. 1c). In order to have a statistically significant number of ICPs, the same RTK GPS was mounted on a snow sled and dragged along the beach while acquiring points at a frequency of 1 Hz (Fig. 1d). The intrusive nature of the sled method to collect ICPs is discussed in “Systematic errors on beach environments.” The locations of GCPs and ICPs are shown in Fig. 2b.

The aerial photos collected during the flights were processed using Agisoft Photoscan Pro (version 1.3.4; www.agisoft.com; now Agisoft Metashape) resulting in orthophotomosaics and digital elevation models (DEM). QGIS was used to crop the

DEMs to the area between the upper part of the swash zone and the initial part of the dune. This allowed to avoid unconformities in the DEM reconstruction due to wave swash in the scene. The DEMs were also cropped to match the least extent flight path acquired with the narrow-angle Canon S100 at 30 m altitude. Matlab® scripts were developed to compare the elevation values of ICPs and the elevation of DEMs at the same points. In general, alignment parameters were set to “high,” dense point cloud generation was executed with “high” settings and depth filtering mode set to “aggressive.” The dense cloud was the source used to build the DEMs with interpolation enabled. All XY coordinates in this study refer to ETRS89/UTM Zone 32N, while heights are given relative to MSL (EGM96). The scripts used are available at DOI: <https://doi.org/10.5281/zenodo.3549893>, and the detailed reports of Agisoft Photoscan are annexed in the [Supplementary Materials](#).

A comparative measure of the efficiency was defined taking into account the following: (i) the placement and GPS positioning of the six GCPs and the pre-flight checks (mobilization time); (ii) flight time (aerial data acquisition time); and

Table 4 Results of the comparison between DEM values and ICP elevations for the entire ICP dataset ($n=2950$ points) and the subset of ICPs included within the area delimited by the GCPs ($n=537$, dashed line in Fig. 2c). *See also right panels in Fig. 4, black lines. **See also right panels in Fig. 4, orange lines. Average and standard deviation values are shown at the bottom of the table in italics

Flight	All ICPs ($n = 2950$)		ICPs Internal to GCPs ($n = 537$)	
	Average DEM – ICP [m]*	RMSE(z) ICPs [m]	Average DEM – ICP [m]**	RMSE(z) [m]
Canon S100 (30 m altitude)	0.015	0.045	0.052	0.055
Canon S100 (40 m altitude)	– 0.023	0.075	0.049	0.053
Canon S100 (50 m altitude)	0.002	0.048	0.050	0.053
GoPro Hero 4 Silver (50 m altitude)	0.044	0.051	0.044	0.046
GoPro Hero 4 Black (50 m altitude)	0.029	0.044	0.047	0.050
Average	<i>0.013</i>	<i>0.052</i>	<i>0.048</i>	<i>0.051</i>
Standard deviation	<i>0.026</i>	<i>0.013</i>	<i>0.003</i>	<i>0.004</i>

(iii) GPS data post process and photogrammetry workflow (elaboration time).

A pre-post storm UAS flight to map the effect of an extreme event was performed using the Canon S100 camera and flying the drone at 50 m above ground level. As, in these flights, a large area was considered, 10 GCPs were placed per flight instead of the 6 GCPs used for the accuracy assessment.

Results

Accuracy assessment

The number of photos collected during the UAS surveys varied from 104 to 217 (the latter for the Go Pro Hero 4 Black, acquiring at 0.4 Hz versus all the other cameras acquiring at 0.2 Hz). Data on the effective overlaps, number of tie points, DEM resolution and total area covered for each flight are reported in Table 3. The effective overlap represents the average number of projections for each point in the sparse point cloud. Tie points are points clearly identifiable in two or more images. They are selected as reference points. The resolution of the DEMs varied between 1.7 and 4.0 cm/pix. The RMSE (z error) varied between 0.006 and 0.016 m.

Figure 2a shows a visualization of a DEM obtained from the flight with the Canon S100 camera flying at a constant altitude of 30 m above ground level. The six GCPs and the 2950 ICPs surveyed with the RTK GPS system are shown in Fig. 2b. The full comparison results and the Agisoft processing reported for each flight are attached as [Supplementary Materials](#) to this paper.

The individual DEMs were compared with the ICPs surveyed with RTK GPS. All results of this comparison are shown in Table 4 and rendered graphically in Fig. 4.

Pre-post storm assessment

Bunkerhill beach is exposed to westerly winds and waves, and beach morphology is mainly driven by storms; also, beach nourishments are carried out on a regular basis. The effect of single storms on beach dynamics is of scientific and management interest (Blossier et al. 2017). In order to assess the morphological effects of a storm event, two surveys were performed on Sept 26th and on October 4th, 2016 at MLW (mean low water) to allow for maximum beach coverage. Details of each flight (including location of GCPs inside the area of interest) are contained in the Agisoft Photoscan reports annexed as [Supplementary Materials](#) to this article.

A storm event started (after 4 days of previously calm wave conditions) on the morning of Sept. 27th, 2016 (Fig. 3d). Wind speed increased from 2.8 m s^{-1} during the survey to ca. 3.8 m s^{-1} for the following days from SWS to W (source: German Meteorological Office DWD). During that time, waves with significant height of 2–3 m (max wave height 3.6 m) were measured offshore for a time span of roughly 72 h. Average wave period increased from 4.3 to 7.4 s (source: Federal Maritime and Hydrographic Agency of Germany (BSH), via COSYNA Data web portal).

The storm event caused changes in the beach morphology, which can be observed in the orthophotomosaics before and after the storm (Fig. 3a, b). The shoreline (countour line at msl extracted from the DEM) evolved from its initial position before the storm (dotted line in Fig. 3) to the post-storm position (continuous line in Fig. 3b). From the comparison of the two orthophotos (Fig. 3a, b), it is possible to discern that the two sandbars in front of the beach migrated northwards by around 30 m. The rip channel between these longshore parallel bars was also wider (and possibly

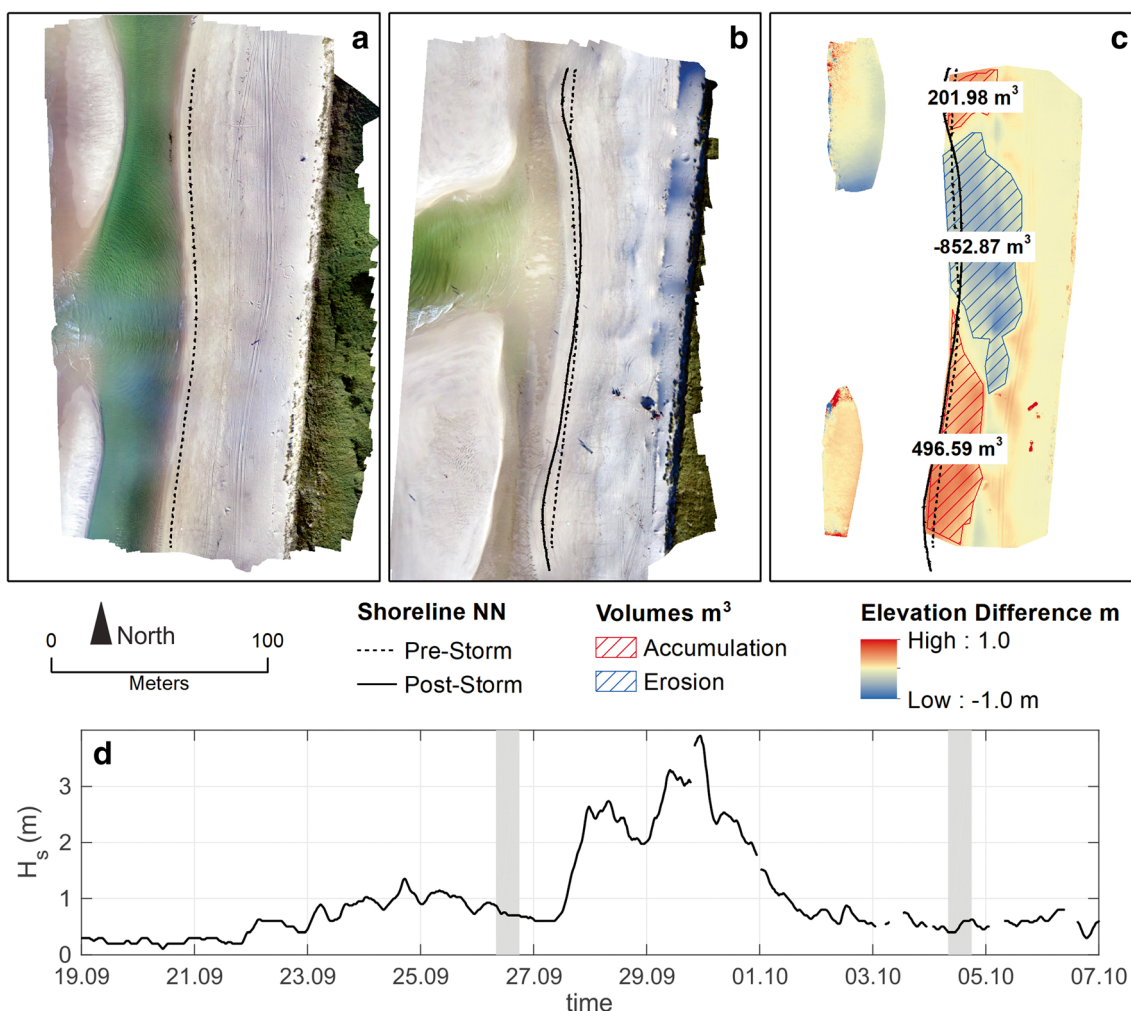


Fig. 3 **a** Orthophotomosaic derived from the survey of the 26th September 2016 and the countourline at msl (dotted line) extracted from the DEM of the same survey (before the storm event). **b** The same from 4th October 2016 and countour lines before (dotted line) and after

(continuous line) the storm event. **c** Difference in elevation between the DEM before and after the storm. **d** Significant wave height (BSH via COSYNA Data web portal). The gray bands represent the pre-post flight times

deeper) after the storm, while the longshore parallel channel between the bars and the coastline appears narrower (and possibly shallower).

Subtracting the pre-post storm DEMs (Fig. 3c), beach morphological changes and areas and volumes of erosion (blue in Fig. 3c) and accretion (red in Fig. 3c) along the beach are measured. The area on the beach around the channel deepened by ca. 40 cm, and the bar located north of the channel was subject to ca. 40 cm erosion. On the contrary, the south part of the beach (south red patch, Fig. 3) and the south bar underwent an aggradation of ca. 40–60 cm. Comparing only the area in common between the two flights, the volume eroded on the beach front (excluding the offshore bars) was ca. 852 m³, in the area in front of the channel separating the two offshore bars. In the areas immediately North and South of this area (Fig. 3c), a deposition of ca. 698 m³ of sand was recorded, with a total loss of sand of ca. 154 m³, over a total surveyed

beach area (excluding the bars) of about 11,000 m². The foot of the dune was not subject to major changes.

Discussion

UAS and survey configuration

The UAS configuration described above is a low-cost solution (see a quantification of the hardware and software costs in Table 5), which should be affordable by most earth science laboratories. The DJI Phantom 2 was first released in December 2013, therefore can be considered at least two generations behind modern drones, but it still represents a reliable low-cost platform able to provide high-resolution and accurate data. In the configuration we used, the drone does not even include a built-in camera connected with the drone electronics. Compared with more modern solutions, this drone presents

Table 5 Quantification of the hardware and software costs. *The cost reported refers to a DJI Phantom 4 pro v.2.0, which is the “DJI Phantom” model in production as of December 2019. The value of the DJI Phantom 2 used in this work is approx. 500 euro and the cost of a Canon S100 is 240 euro. Therefore, the total cost sustained to obtain the results illustrated in this study is approx. 1490 euros. Note: Costs are indicative and refer to market prices at the time of writing (December 2019). Drone costs are extremely variable in time

Description	Approx. Cost (euro)
DJI Phantom*	1700
Tablet	250
Metashape Agisoft Pro Edu	500
Total cost (UAS platform plus photogrammetric software)	2450

the advantage to have a relatively large space under the main body of the copter, which allows mounting different cameras with or without a gimbal. Equipped with a BTU Bluetooth module, the Phantom 2 can be connected to a mobile device where flights can be programmed with the native Phantom DJI app, “DJI Ground Station,” that allows programming a flight path but, as described above, does not allow automated calculation of front and side overlap of the resulting photos. This information can be easily obtained knowing the geometry of the camera sensor and few other parameters as described above.

For this study, six ground control points (GCP) were placed in a 25×40 m area (dashed lines in Fig. 2c). This part of the DEM is referred to as “Internal to the GCPs” in contrast to the entire survey area, located outside of the dashed line in Fig. 2c. In the area internal to GCPs, we placed six targets, four at the edges and two at the center. In an area of similar dimensions, Harwin et al. (2015) used a similar spatial configuration on 5 and 13 GCPs and found a difference of only few millimeters between the final DEMs accuracy (see their Table 3, products “Self13GCP22mm” and “Self5GCP22mm” that are more comparable with our study design). In general, though, the number and location of GCPs should be evaluated before the survey, in order to not underestimate the number of needed GCPs. An example of such evaluation is provided by Laporte-Fauret et al. (2019) who, working on a 350×1000 m beach, stated that “the use of 5 CGPs [...] is the minimum requirement in order to reach satisfactory accuracy with a RMS_Z error.”

Doming of the scene

Mapping the difference between DEM and ICP elevations (Fig. 2c), it is evident that, beyond the extent of the area delimited by the GCPs, the topographic reconstruction is subject to a doming effect (Fig. 4). The reasons for this effect originate in the photogrammetric process, as described by

James and Robson (2014): Structure from motion (SfM) reconstructions of regular linear patterns of images cannot compensate for radial lens distortions accurately when camera self-calibration is necessary. This fact causes a DEM deformation, which can be reduced by adding oblique imagery or with a suitable distribution of control points in the surveyed area.

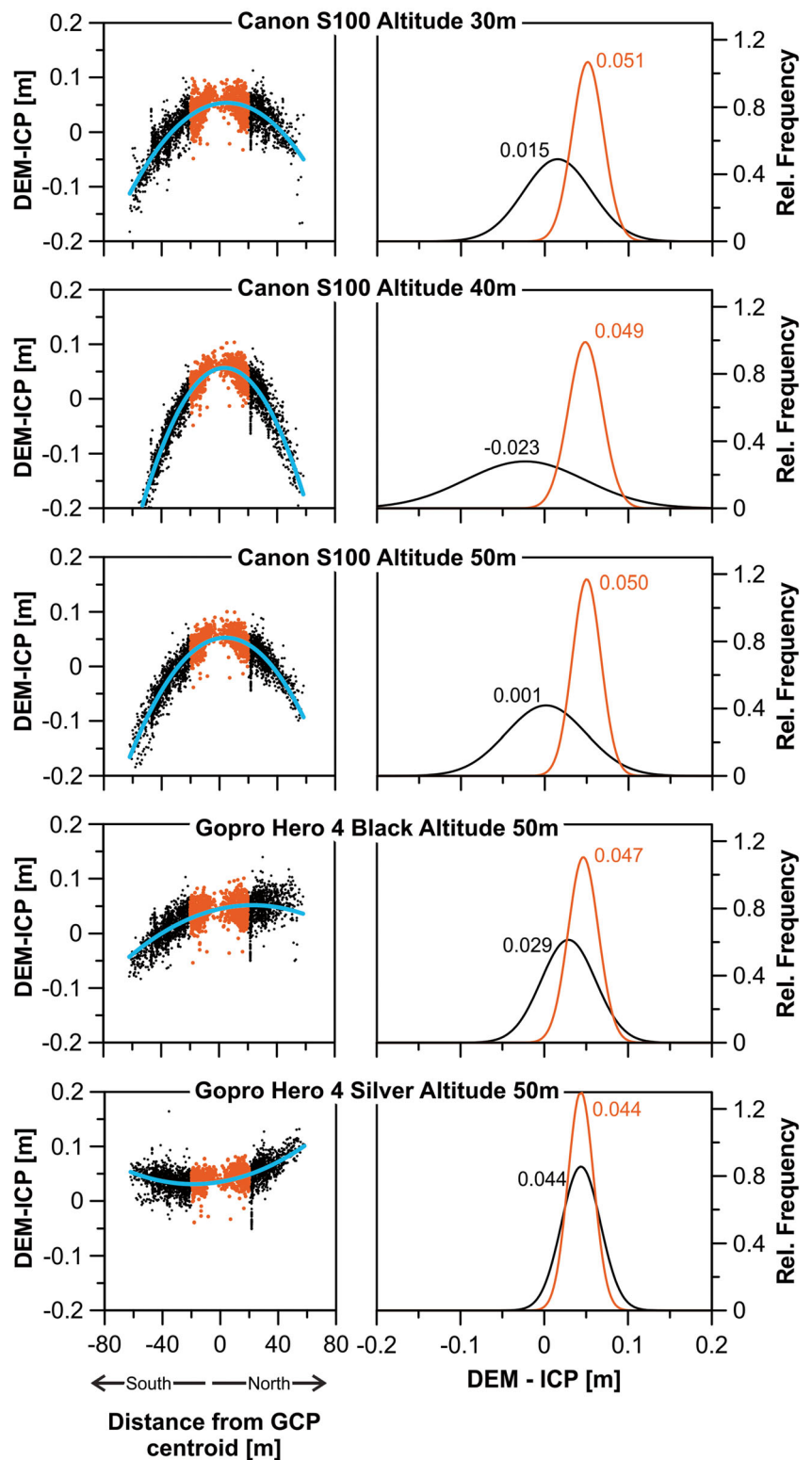
The doming effect on the DEMs is evident in all flight constellations (Fig. 4), and it does not change with altitude when using the Canon S100. The doming appears greatly diminished at 50 m altitude with both GoPro cameras. In the central area of each DEM (dashed lines in Fig. 2c), delimited by the GCPs (orange points in Fig. 3, left panels), the doming is reduced.

As a result of the doming effect, different results must be expected when comparing the entire survey area with the area internal to the GCPs (Table 4). In fact, the average difference between DEMs in the entire study area (Gaussian curves in black, Fig. 4 right panels) is 0.013 ± 0.026 m. For comparison, the average difference between different DEMs when only considering the area internal to the GCPs (Gaussian curves in orange, Fig. 4 right panels) is 0.048 ± 0.003 m. It is important to note that the standard deviation of the DEM-ICP elevation difference is one order of magnitude smaller when considering the area internal to the GCPs. This is also true when comparing the RMSE(z) among different DEMs that has the same average but shows a significantly lower standard deviation between different flight and camera configurations when only the area internal to the GCPs is considered.

Systematic errors on beach environments

Results (Table 4; Fig. 4) indicate that for the area that is not affected by doming (i.e. the area internal to the GCPs), the RMSE(z) between DEM and ICPs elevation are similar for all flight and camera configurations (0.051 ± 0.004 m). DEM obtained from the UAS flights are about 5 cm higher than the ICPs. This is a systematic error, which originates under every survey set-up: GCPs were measured with the RTK-GPS mounted on a survey pole; the ICPs were measured with the GPS mounted on a sled (Fig. 1d). Due to the soft sand, it is possible that the sled was digging few centimeters while being dragged. This interpretation is supported by the circumstance that three GCPs located less than half meter from the closest ICP are systematically higher by a range of 6 and 10 cm. Despite a possible variance in beach surface properties (i.e. wet vs. drier beach and finer vs. coarser sand), leading to different vertical sinking of the sled, this error may be considered as systematic and constant throughout surveys. In spite of this statement, the data presented in this work are not corrected against this systematic error since further research could assess this error based on different sedimentological properties of the beach. Similar systematic errors were already highlighted by Talavera et al. (2018a) and Gonçalves and Henriques (2015)

Fig. 4 Left panels: Plot of DEM-ICP difference versus distance from the GCP centroid, located at the center of the scene. The orange dots indicate the area inside the GCP boundaries. The blue line represents a second-order polynomial trough the points. Right panels: Gaussian distribution of the DEM-ICP difference for the entire scene (black line) or for the area within the GCPs (orange line). Numbers indicate the average DEM-ICP value, in meters



who found, respectively, positive biases of up to 5 cm and up to 7.5 cm in comparing GPS measurement and drone values on sandy beaches. Both studies used similar techniques than those used in this study to measure GCPs and ICPs. For example, Talavera et al. (2018a, b) used a pole to measure

GCPs, but, for the collection of ICPs, the GNSS survey system was instead mounted on a backpack. The weight on the sand of the operator carrying the GPS during the ICP measurements was considered the source of the systematic error following the same rationale we illustrate above. Also Pitman

et al. (2019) highlighted the intrusive nature of the RTK-GNSS methods (vehicle mounted and pedestrian), which alter the beachface by sinking into the sand in contrast to the non-invasive nature of the UAS method.

Efficiency of drone survey

In this study, a DEM covering an area of 1000 m² with a resolution of 1.7 to 4 cm/pix (Table 3) and RMSE(z) of 0.051 ± 0.004 m (Table 4) was obtained. For methodological inter-comparison, survey efficiency can be calculated. The equipment mobilization time was about 80 min, the aerial data acquisition time about 10 min and the time for analysis lasted about 270 min. Thus, the efficiency of this UAS beach survey is about 3 m² min⁻¹. It is noted that this number is highly dependent on the flight settings. If the setup with the Canon S100 (50 m altitude) and an area of 14,600 m² (Table 2) is considered, the survey efficiency changes to ca. 40 m² min⁻¹. The DEM in this case was affected by the doming effect described above, which should be solved placing the GCPs over a wider area, and/or adding GCPs (thus, increasing the mobilization time). The survey efficiency of the UAS-photogrammetry method described here will improve in the near future, thanks to two main technological developments. First, advancements in computer science will cut the photogrammetry processing time needed for large datasets. Second, the availability of low-cost RTK systems on-board drones has already reduced the mobilization time by reducing the number of control points needed to obtain high-accuracy DEMs for large areas (Forlani et al. 2018).

Conclusions

The results allow to highlight different aspects related to the use of UAS and photogrammetry for the monitoring of beach environments.

1. UAS-photogrammetry is used frequently for the measurement of beach topography (Table 1) as it is at once accurate, rapid and low-cost. Several studies report vertical accuracies (RMSE(z)) in the order of a few millimeters to a few decimeters.
2. It is possible to achieve a final DEM accuracy in the order of ~5 cm using a minimal low-cost drone configuration. In this study, results did not change significantly by changing the altitude or the camera type.
3. These results are only valid for the area delimited by the GCPs. Outside of this area, a doming effect related to the photogrammetric processing of UAS photos degrades the quality of the results. This stresses the importance of

careful planning the positioning of GCPs and flight path according to the area of interest to be mapped.

4. Within the area delimited by the GCPs survey (ca. 1000 m²), an efficiency of the DEM generation of ca. 3 m² min⁻¹ has been calculated. This may be regarded as the minimal efficiency of a drone survey on a beach environment.
5. The geometry that was adopted for the emplacement of GCPs has been successful to gather data at very high resolution within the area delimited by the GCPs. A similar GCP pattern was used to map a larger area to assess the pre-post storm morphological variations at a larger scale.
6. The analysis of derived DEMs must consider the survey method for CPs on soft sands. In fact, collecting GCPs with the GNSS mounted on a pole versus mounted on a sled for ICPs may introduce systematic errors that need to be accounted for.
7. It was shown that UAS data analyzed with photogrammetric methods can give information on beach topography with accuracy on the same order of magnitude as the most commonly used monitoring method (i.e. RTK transects). The advantages of the UAS-photogrammetry method reside in the survey efficiency (ca. 3 m² min⁻¹) and in the higher detail with respect to topographic transects, i.e. it allows a full reconstruction of the beach topography.

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Data availability Supplementary Materials annexed to this paper include the reports of the Agisoft Photoscan (now Agisoft Metashape) processing and supplementary figures detailing the results for each drone flight. The Matlab® scripts used to produce the comparison maps on which Figs. 2, 3 and 4 are based are available at this link: <https://doi.org/10.5281/zenodo.3580721>.

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